

TECHNICAL REPORT STANDARD PAGE

1. Report No. FHWA/LA-93/277		2. Government Accession No.	3. Recipient's Catalog No.
4. Title and Subtitle WATER LEVEL STATISTICS FOR DESIGN OF TRANSPORTATION FACILITIES IN COASTAL LOUISIANA		5. REPORT DATE JULY 1993	
		6. PERFORMING ORGANIZATION CODE	
7. Author(s) J. N. Suhayda (1), M. Alawady (1), and B. Naghavi (2)		8. Performing Organization Report No. 277	
9. Performing Organization Name and Address (1) Department of Civil Engineering Louisiana State University Baton Rouge, LA 70803		10. Work Unit No.	
		11. Contract or Grant No. 90-9SS	
12. Sponsoring Agency Name and Address (2) Louisiana Transportation Research Center 4101 Gourrier Avenue Baton Rouge, LA 70808		13. Type of Report and Period Covered Final Report October 1991 - October 1992	
		14. Sponsoring Agency Code	
15. Supplementary Notes Conducted in cooperation with the U.S. Department of Transportation, Federal Highway Administration.			
16. Abstract Transportation facilities in the coastal zone of Louisiana, such as roadways, bridges, and ports, are subject to flooding which must be considered in the design, operation, and maintenance of these facilities. Flooding can result from hurricanes, tidal action, run-off, backwater, and combinations of these events. This report presents the results of a study to determine the hurricane flood elevations in coastal Louisiana which represent average return intervals of 10, 25, 50, 100, and 500 years. Since flood water level statistics are changing in coastal Louisiana due to dredging of channels, levee construction, sea level rise, wetland loss, and subsidence, this study also establishes within LaDOTD the capability for assessing future risk.			
17. Key Words Transportation facilities, coastal zone, flooding, flood level, hurricanes, Hurricane Andrew, tidal action, water level statistics, computer model, Cameron Parish, Plaquemines Parish, FEMA.		18. Distribution Statement No restrictions. This document is available to the public through the National Technical Information Service, Springfield, Virginia 22161.	
19. Security Classif. (of this report) Unclassified	20. Security Classif. (of this page) Unclassified	21. No. of Pages 39	22. Price

**WATER LEVEL STATISTICS FOR DESIGN OF TRANSPORTATION
FACILITIES IN COASTAL LOUISIANA**

Volume 1

FINAL REPORT

by

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STATE PROJECT NO. 736-15-0081
FEDERAL AID PROJECT NO. HPR 0010(16)
LSU PROJECT NO. 127-15-4168

CONDUCTED FOR

LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT
LOUISIANA TRANSPORTATION RESEARCH CENTER
in Cooperation with
U.S. Department of Transportation
FEDERAL HIGHWAY ADMINISTRATION

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July 20, 1993

ABSTRACT

Transportation facilities in the coastal zone of Louisiana, such as roadways, bridges, and ports, are subject to flooding which must be considered in the design, operation, and maintenance of these facilities. Flooding can result from hurricanes, tidal action, run-off, backwater, and combinations of these events. This report presents the results of a study to determine the hurricane flood elevations in coastal Louisiana which represent average return intervals of 10, 25, 50, 100, and 500 years. Since flood water level statistics are changing in coastal Louisiana due to dredging of channels, levee construction, sea level rise, wetland loss, and subsidence, this study also establishes within LaDOTD the capability for assessing future risk.

The research used a computer model and landscape data base to develop the water level statistics for coastal Louisiana. The computer model was developed by the Federal Emergency Management Agency for conducting flood insurance studies. The model incorporates land topography, vegetation, coastal bathymetry, rivers, channels, roadways, ridges, and levees. The response of the coastal and wetland water levels to tides, winds, and river discharge are included in the computations. Data used for input to the model were based upon parish and state surveying data, quadrangle sheets, USGS maps, and NOS charts.

Water level statistics are described on a grid having a spacing of 10,000 feet for the all areas of Louisiana south of latitude 30 degrees 30 minutes (Lambert Y = 670,000 ft). For each grid cell, the annual probability of water levels exceeding given elevations were determined. These statistics were used to compute the flood water elevations. Results of the study are presented as water level elevations having average return periods of 10, 25, 50, 100, and 500 years for each cell.

Results of the study indicate that hurricane flooding affects essentially all locations in coastal Louisiana. The 100-year flood elevations range from 11 feet in Cameron Parish to 13 feet in Plaquemines Parish. The north/south variations in the 100 year elevations indicate, as expected, that the highest flood elevations occur at the coastline. But the area of next highest flooding is at the northern edge of the coastal zone, where elevation can reach 8 to 10 feet. The lowest 100-year flood elevations occur in the midsection of the coastal area. A few sites at the northern edge of the coastal zone experience flooding only during extreme storms. The 50-year flood levels are generally two to three feet less than the 100-year levels. Other flood levels, 25- and 10-year, are generally 5 and 7 feet less than the 100-year elevations, though many locations show different relationships. The flood levels are conservative at this time by about one foot.

The study indicates that hurricane flooding level statistics in coastal Louisiana are highly variable and that they are sensitive to the landscape changes occurring in the state. It is recommended that these statistics be updated periodically to allow the effects of these continuing changes to the landscape and the global sea level rise to be accounted for in the flood threat statistics.

ACKNOWLEDGMENTS

This investigation was sponsored by the Louisiana Transportation Research Center (LTRC) and the Federal Highway Administration under State Project No. 736-15-81, Federal Aid Project No. 0010 (016), and Research Project No. 90-9SS.

The authors of this report acknowledge the support and encouragement of the project review committee: Mr. Peter Allain (Federal Projects Engineering Supervisor), Mr. Henry Barousse (Chief, Water Resources Design & Development) and Mr. Edmond G. Preau (Deputy Director, Public Works & Flood Control).

The authors are also grateful to Mr. Paul M. Griffin, P.E., Geophysical Systems Research Administrator of LTRC for his guidance and support.

IMPLEMENTATION STATEMENT

The results of the completed work consist of water level statistics and a flood prediction model.

The water level statistics are presented in a user's guide (Volume II) with accompanying maps. The users guide and maps can be directly used by non-specialist personnel in LaDOTD to determine the flood elevation at a specific site in coastal Louisiana. The water elevation associated with average return intervals of 10, 25, 50, 100, and 500 years are given in the users guide for grid points indicated on the grid maps. Six grid maps at a scale of 1 to 250,000 are used to locate the site for which water level statistics are desired. Once the location is found on the map, the grid cell containing the site is identified. This cell has coordinates k and j, where k is the column and j the row. This pair of numbers is used in the user guide to find the flood elevation listings for that cell. The user guide lists the flood elevations for the various average return periods in feet relative to National Geodetic Vertical Datum. The users guide has instruction material contained in the introduction.

The hurricane flooding computer model has been transferred directly to LaDOTD and is available for periodic updating of the flood elevations, as well as other uses. The computer model has been extensively documented by the Federal Emergency Management Agency.

The LTRC staff has been directly involved in this research to ensure that the results are compatible with LaDOTD requirements. Questions concerning the user guide, the computer program, and the application of these results to specific LaDOTD projects may be directly addressed to LTRC.

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METRIC CONVERSION FACTORS*

<u>To Convert from</u>	<u>To</u>	<u>Multiply by</u>
<u>Length</u>		
foot	meter (m)	0.3048
inch	millimeter (mm)	25.4
yard	meter (m)	0.9144
mile (statute)	kilometer (km)	1.609
<u>Area</u>		
square foot	square meter (m ²)	0.0929
square inch	square centimeter (cm ²)	6.451
square yard	square meter (m ²)	0.8361
<u>Volume (capacity)</u>		
cubic foot	cubic meter (m ³)	0.02832
gallon (U.S. liquid)**	cubic meter (m ³)	0.003785
gallon (Can. liquid)**	cubic meter (m ³)	0.004546
ounce (U.S. liquid)	cubic centimeter (cm ³)	29.57
<u>Mass</u>		
ounce-mass (avdp)	gram (g)	28.35
pound-mass (avdp)	kilogram (kg)	0.4536
ton (metric)	kilogram (kg)	1000
ton (short, 2000 lbs)	kilogram (kg)	907.2
<u>Mass per Volume</u>		
pound-mass/cubic foot	kilogram/cubic meter (kg/m ³)	16.02
pound-mass/cubic yard	kilogram/cubic meter (kg/m ³)	0.5933
pound-mass/gallon (U.S.)**	kilogram/cubic meter (kg/m ³)	119.8
pound-mass/gallon (Can.)**	kilogram/cubic meter (kg/m ³)	99.78
<u>Temperature</u>		
deg Celsius (C)	kelvin (K)	$t_k = (t_c + 273.15)$
deg Fahrenheit (F)	kelvin (K)	$t_k = (t_f + 459.67)/1.8$
deg Fahrenheit (F)	deg Celsius (C)	$t_c = (t_f - 32)/1.8$

*The reference source for information on SI units and more exact conversion factors is "Metric Practice Guide" ASTM E380.

**One U.S. gallon equals 0.8327 Canadian gallon.

INTRODUCTION

Water level statistics for the design of transportation facilities in coastal Louisiana are needed because this area is subject to flooding by hurricane surges and is undergoing extensive natural and man-made alterations which are causing a modification of the extent of flooding. The use of historic flood levels is misleading in this case since the flood threat is changing with time. This report presents the results of work to provide up-to-date information about hurricane flood water level statistics for coastal Louisiana that reflect present landscape conditions and to provide the ability to easily update flood elevations in the future.

The water level statistics presented in this report are based on a hydrodynamic computer model to compute the threat of hurricane flooding within the coastal zone of Louisiana. Hydrodynamic models of flooding are currently used to determine flood water levels in rivers and channels. The hydrodynamic model used in this study is the overland flooding model of the Federal Emergency Management Agency. This is the same model used to compute the base flood elevations for the National Flood Insurance Program. The model has not been modified in this study, other than to make it more easily operate on a microcomputer. The latest information concerning topographic data and hurricane statistics has been used in the study.

The statistics of the hurricane flood water elevations were defined for several average return periods, i.e., 10, 25, 50, 100, and 500 years. These statistics were determined for locations referenced to Lambert coordinates using a joint probability analysis. In this analysis water levels are forecast for a variety of hurricane storms using the surge simulation computer model.

There has been no previous work on the use of computer model derived hurricane flood statistics for LaDOTD.

geometry, atmospheric pressure and storms, boundary conditions, and bottom friction resistance coefficients. The modelling also included sub-grid barriers such as islands, roadways, levees, and sub-grid channels such as rivers, bayous and canals.

METHODOLOGY

The approach taken in this study was to follow the method recommended by the Federal Emergency Management agency for determining base flood elevations for the Flood Insurance Rate Maps.

The methodology is called the Joint Probability Method (JPM). This method incorporates historical data on representative storm parameters. Statistical distributions of the storm parameters that affect flood levels are then developed. From these distributions, a large population of "synthetic storms" is generated. These storms are called "synthetic" because they resemble historical storms. They could have occurred but may not have been observed. The surge model is then used to determine the storm surge elevations produced along and inland of the coastline of interest for each of these storms. The JPM is used to infer the statistics of these surge levels from the statistics of the meteorological parameters that define the storm. An overview of the JPM follows.

Five storm parameters are used to define synthetic hurricanes. These are the central pressure depression, the radius to maximum winds, storm forward speed, direction of storm motion, and storm track location with respect to the study area. These parameters define the surge-producing potential of a hurricane. The probability distributions of these parameters are derived from a statistical analysis of historical hurricanes that have affected the study area. These probability distributions are then divided into discrete intervals, each interval represented by a single parameter value and an appropriate probability weight. The combination of all discrete parameter values represents a large set, or ensemble, of several synthetic storms.

The actual surge that each synthetic storm produces at a location is determined through detailed hydrodynamic modeling. It is important to realize that the accuracy of the JPM hinges on the use of a simulation model that accurately simulates the surges caused by hurricanes. The required simulation capability actually involves more than one model. One model is used to simulate the hurricane forcing on the ocean. This forcing includes windstress and barometric pressure gradients. Both of these are defined about the center of the hurricane. Their magnitude and areal extent are determined by the central pressure depression and the radius to maximum winds of the storm. The second model is a hydrodynamic model for the area of interest. This model simulates the surge produced by the atmospheric forcing. Surge generation, propagation, and transformation in shallow water are normally modeled using an offshore grid and an inland or nearshore grid.

The peak water-surface elevation that results from the combination of the storm surge with the astronomic tide depends on the magnitude of the astronomic tide and the phasing between the astronomic tide and the storm surge. Because of dynamic coupling, this combination is often non-linear, that is, it is not possible to simply add the computed surge to the known astronomic tide.

The frequency of the storm, and hence the frequency of the storm surge elevation, is defined by the joint probabilities of the storm characteristics. This frequency is computed as the historical density of storms, in events per year per nautical mile, multiplied by the probability of a storm with specific characteristics. These characteristics include the radius to maximum winds, storm speed, central pressure depression, track angle, and storm track. The joint probability of these various parameters is evaluated as the product of the probabilities of each of the storm parameters. When these parameters are statistically dependant, conditional probabilities should be used. The combination of surge with tide is considered to be random, i.e., the surge has an equal probability of occurring at any phase of the tide.

DATA COLLECTION

Data needed as input to set up and run the simulation of the model was acquired from available sources. The methodology for collecting and using the data needed in the study is described below.

Topographic Data

The topographic data used as input to the numerical calculations was based upon USGS quad sheets, NOS bathymetric charts and topographic data taken by parishes and by the state.

The land topographic data was primarily based upon using 7.5' and 15' USGS quad sheets. The quad sheets used cover all pertinent areas of Louisiana, Mississippi and Texas from 88 degrees west longitude to 96 degrees west longitude and from 30 degrees 30 minutes north latitude southward to the shoreline. The quad sheets used were the latest available and have dates ranging from 1934 to 1985, with the majority of the map dates ranging from 1970 to 1985. The maps contain contours at 5 foot intervals and spot measurements to the nearest foot at scattered points. The datum of the quad sheets is NGVD 1929. Additional data was taken from the USGS metric maps series 1: 100,000. These maps contain information from topographic surveys in the period 1971-1973.

The inland and offshore bathymetric data was based upon NOS charts in the 1100 and 11000 series and quad sheets. The charts cover the same longitude limits as the quad sheets and extend in a north/south direction from the shoreline to beyond the edge of the continental shelf. The dates of the charts are 1984, 1985, and 1986. These charts are also used to define the offshore bathymetry for the calibration simulations. The charts contain contours at 1 fathom intervals and a large number of spot water depths. The datum for the charts is Mean Lower Low Water. This datum differs from mean sea level by about .7 to .8 feet.

The topographic data taken by parishes and by the state were also used in setting up the model. These data were located on the appropriate 7.5' quad sheet being used to set-up the model grid. The topographic data was augmented by using existing aerial photos and acquired satellite images that allowed the areal extent of the topographic features to be assessed.

The topographic data within each grid cell were averaged to determine the ground elevation value input into the model. Where topographic data was lacking, the grid cell elevation was assigned based upon vegetation type. It is known that certain types of marsh plants occupy habitats where water level is at a fixed relation to the tidal datums. Most of the salt marsh areas of the state were taken as a few tenths of a foot above mean tide level. The mean tide level itself is at an elevation of about .5 to 1 foot above the present NGVD datum. The datum for the recently surveyed topographic data is NGVD 1982, although some data refers to NGVD 1965. There is a problem with reconciling the various datums used for different data sets, because in Louisiana the bench marks are sinking while sea level is rising and NGVD is being redefined. In this study all elevations refer to the 1982 NGVD datum.

Barrier and River Data

Barriers and rivers which occur in the coastal zone have a controlling influence on flood levels. Barriers include roadways, levees, and natural features such as cheniers. Rivers include channels, canals, and inlets. These features are typically much smaller in width than a grid cell, having widths that are about 100 to 1000 feet. The information needed about barriers is the elevation, width, and roughness. Data was obtained for the sub-grid scale landscape features of barriers and rivers from a variety of sources. The barrier elevations for the inland grid were taken from the maps used to determine topography. These maps contain selected elevations of the ground around the barrier crest. Additional information was obtained from USGS quad sheets which contain elevations for bench marks.

The river data was taken primarily from the NOS charts used for obtaining the bathymetric data. Additional data was obtained from the Corps of Engineers and from professional surveyors.

Hurricane Statistics

The source of the data and methods used to determine the hurricane frequency and parameter statistics was the report by Ho, et al. [1], referred herein to as NWS86. The data base and methodologies presented in NWS86 were developed specifically for flood forecasting studies and therefore the approach taken herein was to follow

NWS86 as closely as was possible. The NWS86 report presents data which describes the statistics of all of the hurricane storm parameters needed for this study.

The hurricane storm parameter statistics used in this study were taken from NWS86. This data from NWS86 was used to determine the cumulative probabilities for the hurricane parameters for various locations along the Louisiana coast. The readings were taken at various miles along the Louisiana coastline from the appropriate graphs and tables in NWS86.

The approach recommended in NWS86 was used to discretize the hurricane parameter probability distributions for use with the surge model. Representative probability ranges were defined for each parameter and the average value of the parameter in the range was computed and taken as the discrete value. The discretized hurricane distributions were determined for various mileages along the coast. Three discrete ranges have been selected for pressure depression, while two ranges have been selected for radius, forward velocity, and direction, and 14 for distance along the coast. The pressure was discretized into three ranges 936, 963, and 991 millibars. These ranges are consistent with the need to represent the lower pressures with higher resolution. The radius was discretized into ranges having average radii of 13 and 39 nautical miles. The forward velocity was discretized into an 8- and 10-knot range. The direction was discretized into two ranges having values of 140 and 205 degrees. This produced hurricane approach directions that are on either side of 180 degrees. Each range had a probability of 50 percent.

SURGE SIMULATION MODEL

The overland flooding model used in the study has been developed by the Federal Emergency Management Agency to predict hurricane flood elevations for the National Flood Insurance Program. The model uses an explicit, two dimensional spaced-staggered, finite difference scheme to simulate the surges caused by hurricanes. Inputs to the model include the bathymetry, coastline configuration, boundary conditions, and bottom friction and other flow resistance coefficients. Also required are the surface wind stress and atmospheric pressure distributions of the hurricane. The surge model simulates the surge elevations everywhere in the modeled region.

The hydrodynamic model uses the principles of conservation of momentum and mass to simulate the response of the ocean to hurricanes. The momentum equation represents a balance between inertial (acceleration) forces and gravity forces, windstress, atmospheric pressure gradient forces, the reactive bottom friction forces, and the Coriolis acceleration effect caused by the earth's rotation. The model uses a rectangular grid to discretize the simulated region of the ocean. The grid is oriented with the y-axis parallel to the general trend of the coastline and the x-axis extending into the ocean. The top of the grid is located where ground elevations are above the expected maximum surge elevation and/or where the inland propagation of the surge through channels becomes negligible. The model can also simulate the flooding of low lying areas resulting from astronomical tides.

In this application of the model two grids were used. An offshore grid having spacing of 30,000 feet extended out to the deep water of the Gulf of Mexico and had lateral boundaries far removed from Louisiana. The purpose of this grid model is to simulate the storm surge generation over the deep ocean and the continental shelf. A second inland grid having a resolution of 10,000 feet was used to generate the surge elevations used in this study. It more accurately represented the nearshore geometry of the study area. The two grids are embedded such that the inland grid covers in more detail a portion of the offshore grid. Water surface elevations at the boundary of the inland grid are transferred from the offshore simulation.

Grids and Input Files

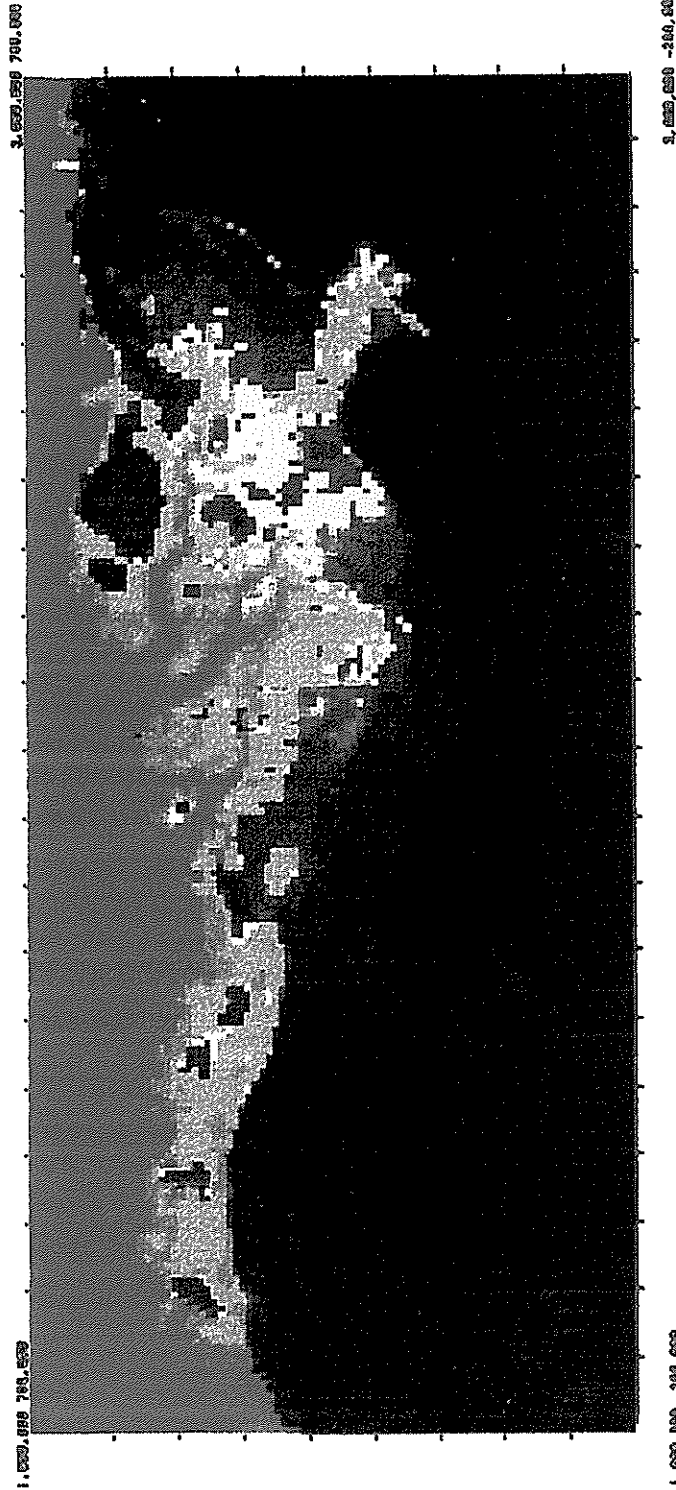
The offshore and inland grids were based upon the Lambert Plane Coordinates (southern grid). The offshore grid is 100 cells east/west by 37 cells north/south and has a grid size of 30,000 ft, as shown in Figure 1. The grid extends from Lambert X = 400,000 on the west to Lambert X = 3,400,000 on the east and extends from Lambert Y = 700,000 on the north to Lambert Y = - 410,000 on the south. The grids lines were oriented along the lines of the Lambert coordinate axis. The inland grid is 170 cells east/west and 59 cells north/south and has a grid size of 10,000 ft, as shown in Figure 2. The grid has a western boundary of Lambert Y = 1,150,000 on the west and Lambert Y = 2,850,000 on the east. The northern border is Lambert Y = 670,000 feet and the southern boundary is Lambert Y = 80,000 feet. The inland grid was separated into two halves during computations.

The topographic and bathymetric data for each grid were obtained from the maps and charts described previously. The average elevation for each 10,000 ft grid cell was based upon averaging several readings over the grid. For the bathymetric data, the actual soundings within each grid cell were numerically averaged. In most cases there were several soundings per grid cell, with some grid cells having 30 to 40 soundings. For the land grid cells, the average elevation was based upon 9 separate elevations; the center, 4 corners, and 4 boundary midpoints. Where survey data was available it was averaged for a 10,000-foot grid cell and used to adjust the grid cell average value obtained from the quad sheets. The 10,000-foot grid averages themselves were averaged over a 30,000-foot grid and were used for the offshore grid. The offshore grid was extended into Texas along lines that parallel the Texas southern grid. The Louisiana and Texas grids were adjusted at the Texas/Louisiana border.

The number of river segments in the model are limited to 300. In order to accurately represent the effect of rivers on flooding, only rivers trending north/south were included in the river input files. Thus certain sections of the Gulf Intercoastal Waterway, which were near the coast and trending in an east west direction, were not included. River data is shown in Figure 3. The barrier data input to the model was for the main roads and levees in the southern part of coastal area of Louisiana. These barriers were placed at the boundaries of the grid cell nearest to their actual location. The barrier data base is shown in Figure 4. The input files for the offshore grid and the inland grid are presented in Tables 1 and 2.

ELEVATION<FT>

- < -256.0
- > -256.0
- > -128.0
- > -64.0
- > -32.0
- > -16.0
- > -8.0
- > -4.0
- > -2.5
- > -2.0
- > -1.5
- > -1.0
- > -0.5
- > 0.0
- > 0.5
- > 1.0
- > 1.5
- > 2.0
- > 4.0
- > 8.0
- > 16.0
- > 32.0
- > 64.0



Gridsize
10,000x10,000

Figure 1
Offshore grid (depth and elevation)

ELEVATION (FT)

- < -256.0
- > -256.0
- > -128.0
- > -64.0
- > -32.0
- > -16.0
- > -8.0
- > -4.0
- > -2.5
- > -2.0
- > -1.5
- > -1.0
- > -0.5
- > 0.0
- > 0.5
- > 1.0
- > 1.5
- > 2.0
- > 4.0
- > 8.0
- > 16.0
- > 32.0
- > 64.0

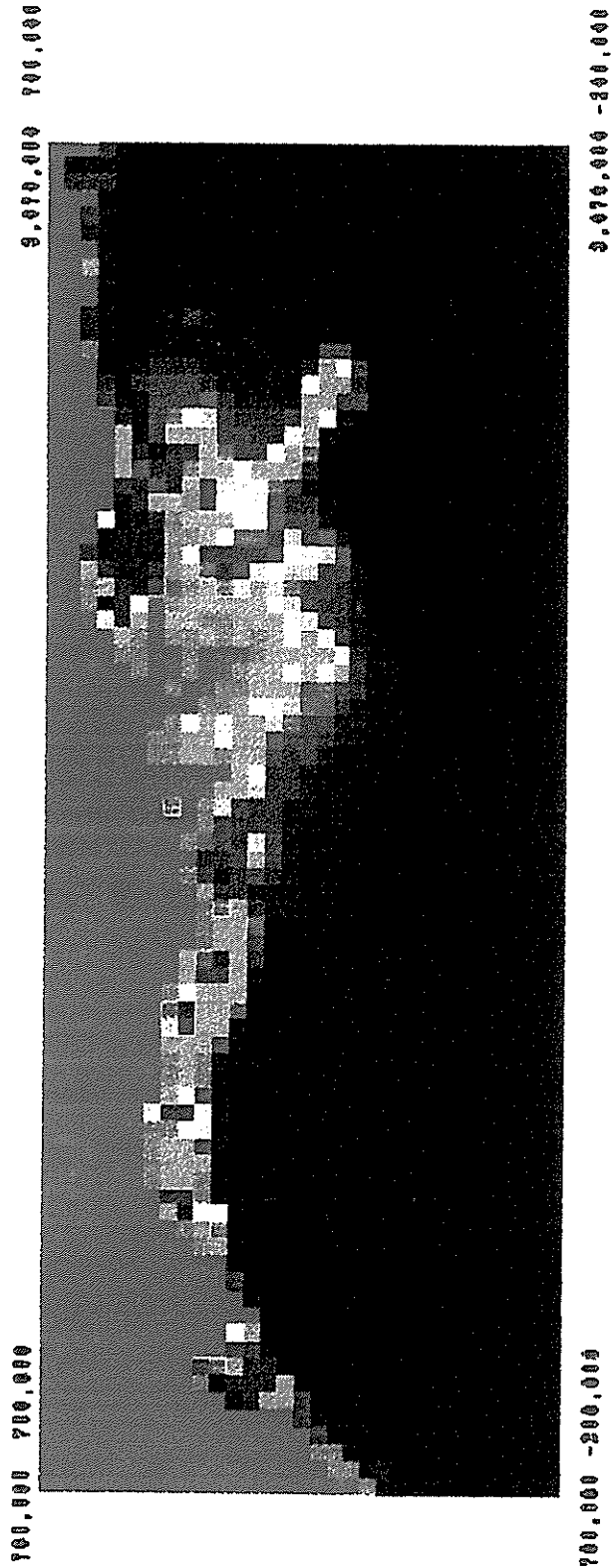
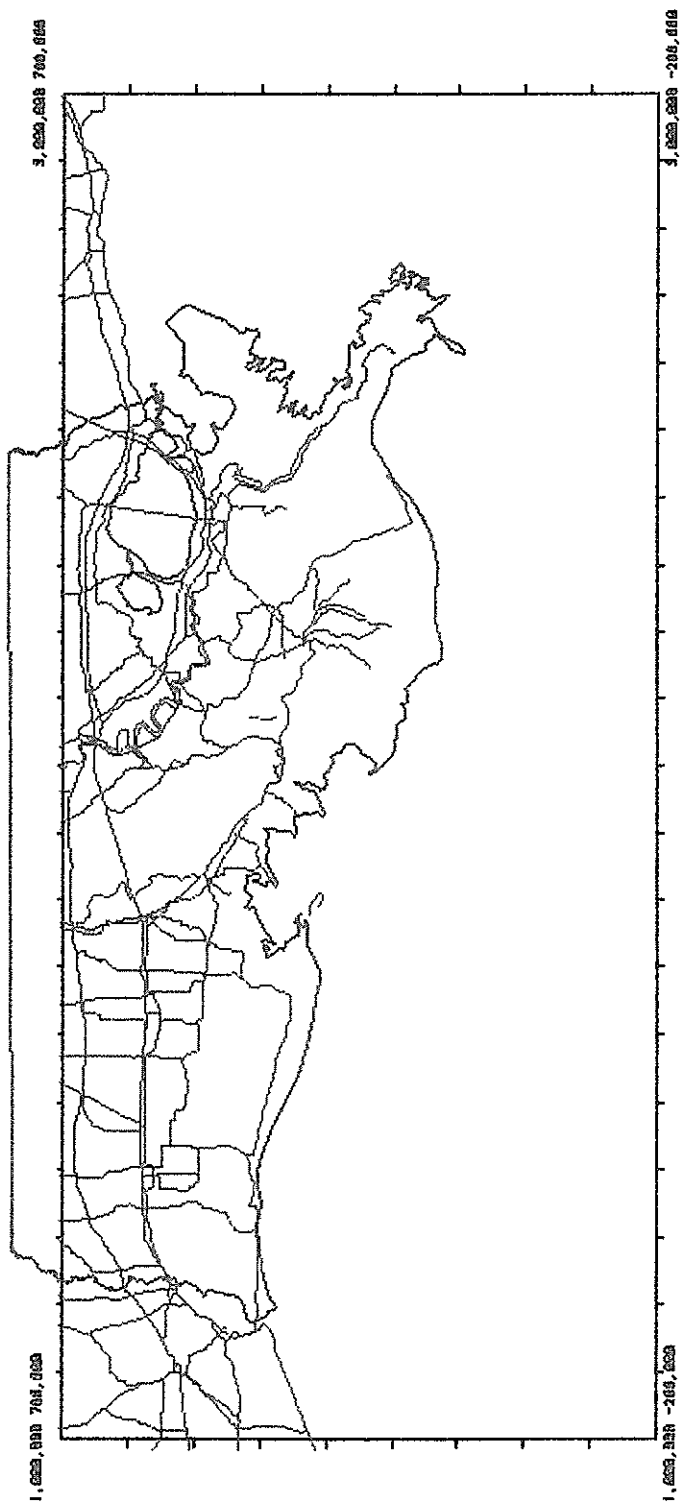


Figure 2
Hurricane coastal flooding simulation

Grid Size
30,000 x 30,000 Ft



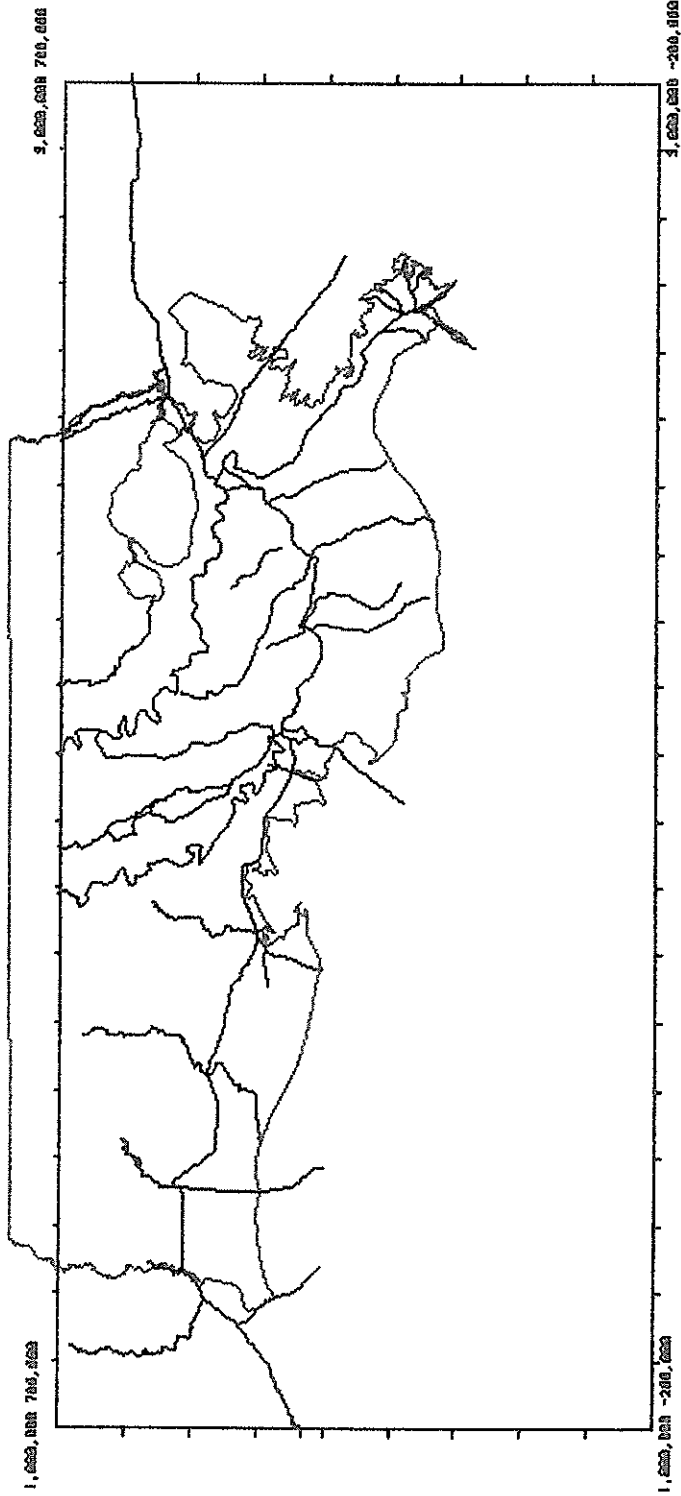
LEGEND

- barrier
- state boundary
- study area
tic marks



Grid size
10,000x10,000

Figure 3
Barrier data



Grindeze
10,000x10,000

Figure 4
River data

Table 1
Input file for the offshore grid

0	100	37	1	1	1	1	1	-13	0	999	0	1	2	0
	0													
	30000.		30000.		300.		.99	172800.	999999.		30.0			
	0.020													
	0		0.00		0.00		0.05	0.00						
	67.0		.900		1.		1.	29.92		0.	180.			
	0.													
	1.1E-6		2.5E-6		14.		.5	75.		0.	0.			
	0.													
	1.0		1.0		1.0		1.0	1.0		1.0	1.0		1.0	
	1.0													
	29.98000		96.42000		0.									
	OFFSHORE -		EAST GRID -											
	0	1	0	0	0	1	0	0	-2	0	0			
	1.00		0.0		25.0		180.0		-15.0		0.035		0.0	
	0.00													
	8	10	-2	2	100	1	37	0						
	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
88	54	14	60	19	68	17	72	12	73	12	73	14	75	4
	4													
	3.2895		220.3947		900.									
	47.6970		220.3947		11		46							
	72.3681		220.3947											
	98.6842		220.3947											
	98.6849		235.1973											
	98.6849		250.0000											
	98.6849		264.8026											
	98.6849		279.6052											
	98.6849		294.4079											
	98.6849		309.2105											
	98.6849		324.0131											
	98.6849		338.8158											
	98.6849		353.6184											
	98.6849		368.4210											
	98.6849		383.2237											
	98.6842		399.6710											
	50.9869		399.6710											
	31.2500		399.6710											
	11.5132		399.6710		4		82							
	999999.													
	0													

Table 2
Input file for the inland grid

110	59	-1	1	1	1	1	0	0	999	0	1	2	0	0	0
10000.		10000.		300.		.99	172800.	999999.		30.0		0.020			
0		0.00		0.0		0.05	0.00								
67.0		0.900		0.		.5	29.92			0.	180.		.7		
1.1E-6		2.5E-6		14.		0.5	75.			0.	0.		0.		
1.0		1.0		1.0		1.0	1.0			1.0	1.0		1.0		
HURRICANE ANDREW -															
0	1	0	188	170	1	1	0	-2	0	1	0	0	1900	7/20/93	
1.00		0.0		5.0		60.0		-25.0		0.033		0.0		0.0	
16	60	1	1	110	1	59	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
19	32	20	29	24	31	29	30	44	40	57	41	60	29	63	32
64	6	67	30	69	29	70	35	74	35	80	33	82	32	82	37
5	21		8.2		0.		0.		100.		5000.	.025	.06		0.
6	21		8.2		0.		0.		100.		5000.	.025	.06		0.
7	21		8.2		0.		0.		100.		5000.	.025	.06		0.
8	20		8.2		0.		0.		100.		5000.	.025	.06		0.
9	20		8.2		0.		0.		100.		5000.	.025	.06		0.
9	30		6.6		0.		0.		100.		5000.	.025	.06		0.
10	21		8.2		0.		0.		100.		5000.	.025	.06		0.
10	29		6.6		0.		0.		100.		5000.	.025	.06		0.
11	21		8.2		0.		0.		100.		5000.	.025	.06		0.
11	25		8.2		0.		0.		100.		5000.	.025	.06		0.
11	28		6.6		0.		0.		100.		5000.	.025	.06		0.
12	22		6.6		0.		0.		100.		5000.	.025	.06		0.
12	27		6.6		0.		0.		100.		5000.	.025	.06		0.
13	22		6.6		0.		0.		100.		5000.	.025	.06		0.
13	27		6.6		0.		0.		100.		5000.	.025	.06		0.
14	23		6.6		0.		0.		100.		5000.	.025	.06		0.
14	27		6.6		0.		0.		100.		5000.	.025	.06		0.
15	24		6.6		0.		0.		100.		5000.	.025	.06		0.
15	26		6.6		0.		0.		100.		5000.	.025	.06		0.
16	24		6.6		0.		0.		100.		5000.	.025	.06		0.
17	25		6.6		0.		0.		100.		5000.	.025	.06		0.
18	25		6.6		0.		0.		100.		5000.	.025	.06		0.
19	26		6.6		0.		0.		100.		5000.	.025	.06		0.
19	33		6.6		0.		0.		100.		5000.	.025	.06		0.
19	34		6.6		0.		0.		100.		5000.	.025	.06		0.
20	27		6.6		0.		0.		100.		5000.	.025	.06		0.
20	32		6.6		0.		0.		100.		5000.	.025	.06		0.
21	28		6.6		0.		0.		100.		5000.	.025	.06		0.
21	31		6.6		0.		0.		100.		5000.	.025	.06		0.
22	28		6.6		0.		0.		100.		5000.	.025	.06		0.
22	30		6.6		0.		0.		100.		5000.	.025	.06		0.
23	28		6.6		0.		0.		100.		5000.	.025	.06		0.
24	31		6.6		0.		0.		100.		5000.	.025	.06		0.
25	31		6.6		0.		0.		100.		5000.	.025	.06		0.
26	31		6.6		0.		0.		100.		5000.	.025	.06		0.
27	31		6.6		0.		0.		100.		5000.	.025	.06		0.
28	31		6.6		0.		0.		100.		5000.	.025	.06		0.
29	31		6.6		0.		0.		100.		5000.	.025	.06		0.
30	25		6.6		0.		0.		100.		5000.	.025	.06		0.
30	30		6.6		0.		0.		100.		5000.	.025	.06		0.
30	31		6.6		0.		0.		100.		5000.	.025	.06		0.
31	31		6.6		0.		0.		100.		5000.	.025	.06		0.
32	31		6.6		0.		0.		100.		5000.	.025	.06		0.
33	32		6.6		0.		0.		100.		5000.	.025	.06		0.
34	32		6.6		0.		0.		100.		5000.	.025	.06		0.
35	32		6.6		0.		0.		100.		5000.	.025	.06		0.
36	30		6.6		0.		0.		100.		5000.	.025	.06		0.
37	31		6.6		0.		0.		100.		5000.	.025	.06		0.
37	32		6.6		0.		0.		100.		5000.	.025	.06		0.

38	31	6.6	0.	0.	100.	5000.	.025	.06	0.
38	33	6.6	0.	0.	100.	5000.	.025	.06	0.
38	54	2.0	0.	0.	2000.	5000.	.025	.06	0.
39	30	5.0	0.	0.	200.	5000.	.025	.06	0.
39	33	6.6	0.	0.	200.	5000.	.025	.06	0.
39	54	2.0	0.	0.	2000.	5000.	.025	.06	0.
40	30	5.0	0.	0.	200.	5000.	.025	.06	0.
40	34	5.0	0.	0.	200.	5000.	.025	.06	0.
41	29	5.0	0.	0.	100.	5000.	.025	.06	0.
41	34	5.0	0.	0.	100.	5000.	.025	.06	0.
42	35	5.0	0.	0.	100.	5000.	.025	.06	0.
43	23	5.0	0.	0.	100.	5000.	.025	.06	0.
43	35	5.0	0.	0.	100.	5000.	.025	.06	0.
44	23	5.0	0.	0.	100.	5000.	.025	.06	0.
44	29	5.0	0.	0.	200.	5000.	.025	.06	0.
44	35	5.0	0.	0.	200.	5000.	.025	.06	0.
44	54	3.0	0.	0.	2000.	5000.	.025	.06	0.
45	22	5.0	0.	0.	200.	5000.	.025	.06	0.
45	29	5.0	0.	0.	200.	5000.	.025	.06	0.
45	35	5.0	0.	0.	200.	5000.	.025	.06	0.
45	54	3.0	0.	0.	2000.	5000.	.025	.06	0.
46	29	6.6	0.	0.	2000.	5000.	.025	.06	0.
46	35	6.6	0.	0.	2000.	5000.	.025	.06	0.
46	38	6.6	0.	0.	2000.	5000.	.025	.06	0.
46	44	5.0	0.	0.	2000.	5000.	.025	.06	0.
46	54	3.0	0.	0.	2000.	5000.	.025	.06	0.
47	29	5.0	0.	0.	2000.	5000.	.025	.06	0.
47	35	5.0	0.	0.	2000.	5000.	.025	.06	0.
47	45	5.0	0.	0.	2000.	5000.	.025	.06	0.
47	54	2.0	0.	0.	2000.	5000.	.025	.06	0.
48	29	5.0	0.	0.	200.	5000.	.025	.06	0.
48	36	5.0	0.	0.	200.	5000.	.025	.06	0.
48	43	5.0	0.	0.	200.	5000.	.025	.06	0.
49	30	5.0	0.	0.	100.	5000.	.025	.06	0.
49	36	5.0	0.	0.	100.	5000.	.025	.06	0.
49	40	5.0	0.	0.	200.	5000.	.025	.06	0.
50	18	25.0	0.	0.	100.	5000.	.025	.06	0.
50	30	5.0	0.	0.	100.	5000.	.025	.06	0.
50	31	5.0	0.	0.	100.	5000.	.025	.06	0.
50	36	5.0	0.	0.	100.	5000.	.025	.06	0.
51	18	25.0	0.	0.	100.	5000.	.025	.06	0.
51	28	5.0	0.	0.	100.	5000.	.025	.06	0.
51	33	5.0	0.	0.	100.	5000.	.025	.06	0.
51	36	5.0	0.	0.	100.	5000.	.025	.06	0.
51	53	4.0	0.	0.	2000.	5000.	.025	.06	0.
52	18	25.0	0.	0.	100.	5000.	.025	.06	0.
52	27	5.0	0.	0.	100.	5000.	.025	.06	0.
52	34	5.0	0.	0.	100.	5000.	.025	.06	0.
52	36	5.0	0.	0.	100.	5000.	.025	.06	0.
52	53	4.0	0.	0.	2000.	5000.	.025	.06	0.
53	20	25.0	0.	0.	100.	5000.	.025	.06	0.
53	26	5.0	8.	2000.	100.	5000.	.025	.06	0.
53	35	5.0	0.	0.	100.	5000.	.025	.06	0.
53	36	5.0	0.	0.	100.	5000.	.025	.06	0.
53	54	5.0	0.	0.	2000.	5000.	.025	.06	0.
54	20	25.0	0.	0.	100.	5000.	.025	.06	0.
54	24	5.0	0.	0.	100.	5000.	.025	.06	0.
54	35	5.0	0.	0.	100.	5000.	.025	.06	0.
54	36	5.0	0.	0.	100.	5000.	.025	.06	0.
54	54	5.0	0.	0.	2000.	5000.	.025	.06	0.
55	21	25.0	0.	0.	100.	5000.	.025	.06	0.
55	23	5.0	0.	0.	100.	5000.	.025	.06	0.
55	35	5.0	0.	0.	100.	5000.	.025	.06	0.
55	36	5.0	0.	0.	100.	5000.	.025	.06	0.
56	22	25.0	0.	0.	100.	5000.	.025	.06	0.
56	23	5.0	0.	0.	100.	5000.	.025	.06	0.
56	35	5.0	0.	0.	100.	5000.	.025	.06	0.
57	22	20.0	0.	0.	100.	5000.	.025	.06	0.
57	36	5.0	0.	0.	100.	5000.	.025	.06	0.

57	54	5.0	0.	0.	2000.	5000.	.025	.06	0.
58	22	20.0	0.	0.	2000.	5000.	.025	.06	0.
58	39	5.0	0.	0.	2000.	5000.	.025	.06	0.
58	53	5.0	0.	0.	2000.	5000.	.025	.06	0.
59	21	20.0	0.	0.	2000.	5000.	.025	.06	0.
59	41	5.0	0.	0.	2000.	5000.	.025	.06	0.
60	21	20.0	0.	0.	2000.	5000.	.025	.06	0.
60	44	5.0	0.	0.	2000.	5000.	.025	.06	0.
61	22	20.0	0.	0.	2000.	5000.	.025	.06	0.
61	46	5.0	0.	0.	2000.	5000.	.025	.06	0.
62	22	20.0	0.	0.	2000.	5000.	.025	.06	0.
63	23	20.0	0.	0.	2000.	5000.	.025	.06	0.
64	23	20.0	0.	0.	2000.	5000.	.025	.06	0.
64	28	5.0	0.	0.	100.	5000.	.025	.06	0.
64	29	5.0	0.	0.	100.	5000.	.025	.06	0.
64	49	5.0	0.	0.	100.	5000.	.025	.06	0.
65	23	20.0	0.	0.	100.	5000.	.025	.06	0.
65	49	5.0	0.	0.	100.	5000.	.025	.06	0.
66	23	20.0	0.	0.	100.	5000.	.025	.06	0.
66	48	9.0	0.	0.	100.	5000.	.025	.06	0.
67	22	17.0	0.	0.	100.	5000.	.025	.06	0.
67	48	9.0	10.	3000.	100.	5000.	.025	.06	0.
68	18	9.0	0.	0.	100.	5000.	.025	.06	0.
68	25	13.0	0.	0.	100.	5000.	.025	.06	0.
68	30	13.0	0.	0.	100.	5000.	.025	.06	0.
68	47	9.0	0.	0.	100.	5000.	.025	.06	0.
69	18	9.0	0.	0.	100.	5000.	.025	.06	0.
69	21	13.0	0.	0.	100.	5000.	.025	.06	0.
69	24	13.0	0.	0.	100.	5000.	.025	.06	0.
69	32	13.0	0.	0.	100.	5000.	.025	.06	0.
69	47	9.0	0.	0.	100.	5000.	.025	.06	0.
70	18	9.0	0.	0.	100.	5000.	.025	.06	0.
70	21	13.0	0.	0.	100.	5000.	.025	.06	0.
70	24	13.0	0.	0.	100.	5000.	.025	.06	0.
70	33	13.0	0.	0.	100.	5000.	.025	.06	0.
71	18	9.0	0.	0.	100.	5000.	.025	.06	0.
71	22	13.0	0.	0.	100.	5000.	.025	.06	0.
71	24	9.0	0.	0.	100.	5000.	.025	.06	0.
71	34	13.0	0.	0.	100.	5000.	.025	.06	0.
72	17	9.0	0.	0.	100.	5000.	.025	.06	0.
72	24	9.0	0.	0.	100.	5000.	.025	.06	0.
72	34	13.0	0.	0.	100.	5000.	.025	.06	0.
73	17	9.0	0.	0.	100.	5000.	.025	.06	0.
73	24	9.0	0.	0.	100.	5000.	.025	.06	0.
73	34	13.0	0.	0.	100.	5000.	.025	.06	0.
74	17	9.0	0.	0.	100.	5000.	.025	.06	0.
74	24	9.0	0.	0.	100.	5000.	.025	.06	0.
74	24	9.0	0.	0.	100.	5000.	.025	.06	0.
74	35	13.0	0.	0.	100.	5000.	.025	.06	0.
75	16	9.0	0.	0.	100.	5000.	.025	.06	0.
75	24	9.0	0.	0.	100.	5000.	.025	.06	0.
75	36	13.0	0.	0.	100.	5000.	.025	.06	0.
76	13	9.0	0.	0.	100.	5000.	.025	.06	0.
76	25	9.0	0.	0.	100.	5000.	.025	.06	0.
76	37	13.0	0.	0.	100.	5000.	.025	.06	0.
77	12	9.0	0.	0.	100.	5000.	.025	.06	0.
77	25	9.0	0.	0.	100.	5000.	.025	.06	0.
77	38	13.0	0.	0.	100.	5000.	.025	.06	0.
78	11	13.0	0.	0.	100.	5000.	.025	.06	0.
78	39	13.0	0.	0.	100.	5000.	.025	.06	0.
79	39	13.0	0.	0.	100.	5000.	.025	.06	0.
80	39	13.0	0.	0.	100.	5000.	.025	.06	0.
81	42	13.0	0.	0.	100.	5000.	.025	.06	0.
82	43	13.0	0.	0.	100.	5000.	.025	.06	0.
83	43	13.0	0.	0.	100.	5000.	.025	.06	0.
84	43	13.0	0.	0.	100.	5000.	.025	.06	0.
85	43	13.0	0.	0.	100.	5000.	.025	.06	0.
86	43	13.0	0.	0.	100.	5000.	.025	.06	0.
87	44	13.0	0.	0.	100.	5000.	.025	.06	0.

46	41	5.0	0.	0.	100.	5000.	.025	.06	0.
46	42	5.0	0.	0.	100.	5000.	.025	.06	0.
46	43	5.0	0.	0.	100.	5000.	.025	.06	0.
47	34	6.6	0.	0.	100.	5000.	.025	.06	0.
47	35	6.6	0.	0.	100.	5000.	.025	.06	0.
47	36	6.6	0.	0.	100.	5000.	.025	.06	0.
47	37	6.6	0.	0.	100.	5000.	.025	.06	0.
47	44	5.0	0.	0.	100.	5000.	.025	.06	0.
48	35	5.0	0.	0.	100.	5000.	.025	.06	0.
48	43	5.0	0.	0.	100.	5000.	.025	.06	0.
48	44	5.0	0.	0.	100.	5000.	.025	.06	0.
48	45	5.0	0.	0.	100.	5000.	.025	.06	0.
48	46	5.0	0.	0.	100.	5000.	.025	.06	0.
49	29	5.0	0.	0.	100.	5000.	.025	.06	0.
49	40	6.6	0.	0.	100.	5000.	.025	.06	0.
49	41	6.6	0.	0.	100.	5000.	.025	.06	0.
49	42	6.6	0.	0.	100.	5000.	.025	.06	0.
50	30	5.0	0.	0.	100.	5000.	.025	.06	0.
50	36	5.0	0.	0.	100.	5000.	.025	.06	0.
50	37	5.0	0.	0.	100.	5000.	.025	.06	0.
50	38	5.0	0.	0.	100.	5000.	.025	.06	0.
50	39	5.0	0.	0.	100.	5000.	.025	.06	0.
51	28	5.0	0.	0.	100.	5000.	.025	.06	0.
51	31	5.0	0.	0.	100.	5000.	.025	.06	0.
51	32	5.0	0.	0.	100.	5000.	.025	.06	0.
52	27	5.0	0.	0.	100.	5000.	.025	.06	0.
52	33	5.0	0.	0.	100.	5000.	.025	.06	0.
53	26	5.0	0.	0.	100.	5000.	.025	.06	0.
53	18	25.0	0.	0.	100.	5000.	.025	.06	0.
53	19	25.0	0.	0.	100.	5000.	.025	.06	0.
53	34	5.0	0.	0.	100.	5000.	.025	.06	0.
53	53	5.0	0.	0.	2000.	5000.	.025	.06	0.
54	24	5.0	0.	0.	100.	5000.	.025	.06	0.
54	25	5.0	0.	0.	100.	5000.	.025	.06	0.
55	20	25.0	0.	0.	100.	5000.	.025	.06	0.
55	23	5.0	0.	0.	100.	5000.	.025	.06	0.
56	21	25.0	0.	0.	100.	5000.	.025	.06	0.
56	35	5.0	0.	0.	100.	5000.	.025	.06	0.
57	22	5.0	0.	0.	100.	5000.	.025	.06	0.
57	35	5.0	0.	0.	100.	5000.	.025	.06	0.
58	35	5.0	0.	0.	100.	5000.	.025	.06	0.
58	37	6.6	0.	0.	100.	5000.	.025	.06	0.
58	38	6.6	0.	0.	100.	5000.	.025	.06	0.
58	53	5.0	0.	0.	2000.	5000.	.025	.06	0.
59	21	20.0	0.	0.	100.	5000.	.025	.06	0.
59	36	5.0	0.	0.	100.	5000.	.025	.06	0.
59	37	5.0	0.	0.	100.	5000.	.025	.06	0.
59	38	5.0	0.	0.	100.	5000.	.025	.06	0.
59	39	5.0	0.	0.	100.	5000.	.025	.06	0.
59	40	5.0	0.	0.	100.	5000.	.025	.06	0.
60	41	5.0	0.	0.	100.	5000.	.025	.06	0.
60	42	5.0	0.	0.	100.	5000.	.025	.06	0.
60	43	5.0	0.	0.	100.	5000.	.025	.06	0.
61	21	20.0	0.	0.	100.	5000.	.025	.06	0.
61	44	5.0	0.	0.	100.	5000.	.025	.06	0.
61	45	5.0	0.	0.	100.	5000.	.025	.06	0.
62	46	5.0	0.	0.	100.	5000.	.025	.06	0.
62	47	5.0	0.	0.	100.	5000.	.025	.06	0.
62	48	5.0	0.	0.	100.	5000.	.025	.06	0.
62	49	5.0	0.	0.	100.	5000.	.025	.06	0.
63	22	17.0	0.	0.	100.	5000.	.025	.06	0.
64	49	5.0	0.	0.	100.	5000.	.025	.06	0.
65	23	5.0	0.	0.	100.	5000.	.025	.06	0.
65	24	5.0	0.	0.	100.	5000.	.025	.06	0.
65	25	5.0	0.	0.	100.	5000.	.025	.06	0.
65	26	5.0	0.	0.	100.	5000.	.025	.06	0.
65	27	5.0	0.	0.	100.	5000.	.025	.06	0.
65	29	5.0	0.	0.	100.	5000.	.025	.06	0.
65	30	5.0	0.	0.	100.	5000.	.025	.06	0.

66	48	5.0	0.	0.	100.	5000.	.025	.06	0.
68	26	13.0	0.	0.	100.	5000.	.025	.06	0.
68	27	13.0	0.	0.	100.	5000.	.025	.06	0.
68	28	13.0	0.	0.	100.	5000.	.025	.06	0.
68	29	13.0	0.	0.	100.	5000.	.025	.06	0.
68	47	9.0	0.	0.	100.	5000.	.025	.06	0.
69	30	13.0	0.	0.	100.	5000.	.025	.06	0.
69	31	13.0	0.	0.	100.	5000.	.025	.06	0.
69	47	9.0	0.	0.	100.	5000.	.025	.06	0.
70	32	13.0	0.	0.	100.	5000.	.025	.06	0.
71	21	13.0	0.	0.	100.	5000.	.025	.06	0.
71	33	13.0	0.	0.	100.	5000.	.025	.06	0.
72	17	9.0	0.	0.	100.	5000.	.025	.06	0.
72	22	13.0	0.	0.	100.	5000.	.025	.06	0.
72	23	13.0	0.	0.	100.	5000.	.025	.06	0.
74	34	13.0	0.	0.	100.	5000.	.025	.06	0.
75	16	9.0	0.	0.	100.	5000.	.025	.06	0.
75	35	13.0	0.	0.	100.	5000.	.025	.06	0.
76	13	9.0	0.	0.	100.	5000.	.025	.06	0.
76	14	9.0	0.	0.	100.	5000.	.025	.06	0.
76	15	9.0	0.	0.	100.	5000.	.025	.06	0.
76	36	13.0	0.	0.	100.	5000.	.025	.06	0.
77	12	9.0	0.	0.	100.	5000.	.025	.06	0.
77	37	13.0	0.	0.	100.	5000.	.025	.06	0.
78	11	9.0	0.	0.	100.	5000.	.025	.06	0.
78	38	13.0	0.	0.	100.	5000.	.025	.06	0.
81	39	13.0	0.	0.	100.	5000.	.025	.06	0.
81	40	13.0	0.	0.	100.	5000.	.025	.06	0.
81	41	13.0	0.	0.	100.	5000.	.025	.06	0.
82	42	13.0	0.	0.	100.	5000.	.025	.06	0.
87	43	13.0	0.	0.	100.	5000.	.025	.06	0.
88	44	13.0	0.	0.	100.	5000.	.025	.06	0.
89	45	13.0	0.	0.	100.	5000.	.025	.06	0.
30.57466	92.11316	0.0000							
15.0	0.5	12							
Bernstein		10000.		10.	0.	2	0		
	8.0	100.0	0.0		.020	64	32		
	8.0	100.0	0.0		.020	63	32		
Harvey		10000.		10.	0.	4	0		
	15.0	200.0	0.0		.020	64	36		
	15.0	200.0	0.0		.020	64	35		
	15.0	200.0	0.0		.020	64	34		
	15.0	200.0	0.0		.020	64	33		
B. Perot		10000.		10.	0.	3	0		
	15.0	500.0	0.0		.020	63	31		
	15.0	500.0	0.0		.020	62	30		
	15.0	500.0	0.0		.020	61	30		
Little Lk		10000.		10.	0.	2	0		
	25.0	500.0	0.0		.020	62	35		
	20.0	500.0	0.0		.020	61	34		
Rigolets		10000.		10.	0.	6	0		
	25.0	2000.0	0.0		.020	80	14		
	25.0	2000.0	0.0		.020	79	13		
	25.0	2000.0	0.0		.020	78	13		
	25.0	2000.0	0.0		.020	77	13		
	25.0	2000.0	0.0		.020	76	13		
	25.0	2000.0	0.0		.020	75	13		
ICWW		10000.		10.	0.	15	0		
	12.0	200.0	0.0		.020	45	34		
	12.0	200.0	0.0		.020	46	33		
	12.0	200.0	0.0		.020	47	34		
	12.0	200.0	0.0		.020	48	34		
	12.0	200.0	0.0		.020	49	34		
	12.0	200.0	0.0		.020	50	35		
	12.0	200.0	0.0		.020	51	35		
	12.0	200.0	0.0		.020	52	35		
	12.0	200.0	0.0		.020	53	35		
	12.0	200.0	0.0		.020	54	35		
	12.0	200.0	0.0		.020	55	35		

	12.0	200.0	0.0	.020	56	34		
	12.0	200.0	0.0	.020	57	33		
	12.0	200.0	0.0	.020	57	32		
	12.0	200.0	0.0	.020	58	31		
Atchaf		10000.	50000.		0.	31		0
	18.0	400.0	0.0	.030	19	48		
	18.0	400.0	0.0	.030	20	47		
	18.0	400.0	0.0	.030	21	46		
	18.0	400.0	0.0	.030	21	45		
	18.0	400.0	0.0	.030	22	44		
	18.0	400.0	0.0	.030	23	43		
	17.0	400.0	0.0	.030	24	42		
	17.0	400.0	0.0	.030	25	41		
	17.0	400.0	0.0	.030	25	40		
	17.0	400.0	0.0	.030	26	39		
	29.0	2000.0	0.0	.030	27	38		
	30.0	2000.0	0.0	.030	27	37		
	34.0	2000.0	0.0	.030	27	36		
	35.0	2000.0	0.0	.030	28	36		
	30.0	1500.0	0.0	.030	29	35		
	32.0	1500.0	0.0	.030	28	35		
	30.0	2000.0	0.0	.030	28	34		
	30.0	1250.0	0.0	.030	28	33		
	30.0	1250.0	0.0	.030	28	32		
	30.0	1500.0	0.0	.030	29	31		
	30.0	1500.0	0.0	.030	29	30		
	30.0	500.0	0.0	.030	29	29		
	30.0	1000.0	0.0	.030	29	28		
	30.0	1000.0	0.0	.030	28	28		
	30.0	1500.0	0.0	.030	27	28		
	20.0	1500.0	0.0	.030	26	28		
	12.0	1800.0	0.0	.030	25	27		
	12.0	1800.0	0.0	.030	24	26		
	12.0	1800.0	0.0	.030	24	25		
	12.0	1800.0	0.0	.030	23	24		
	12.0	1800.0	0.0	.030	22	23		
Barataria		10000.	100.		0.	21		0
	25.0	3500.0	0.0	0.020	70	46		
	25.0	3500.0	0.0	0.020	69	45		
	9.0	1000.0	0.0	0.020	68	44		
	9.0	1000.0	0.0	0.020	68	43		
	9.0	1000.0	0.0	0.020	68	42		
	9.0	1000.0	0.0	0.020	68	41		
	9.0	1000.0	0.0	0.020	68	40		
	9.0	1000.0	0.0	0.020	68	39		
	9.0	1000.0	0.0	0.020	68	38		
	9.0	1000.0	0.0	0.020	67	37		
	9.0	1000.0	0.0	0.020	67	36		
	9.0	1000.0	0.0	0.020	67	35		
	9.0	1000.0	0.0	0.020	66	34		
	9.0	1000.0	0.0	0.020	65	33		
	9.0	1000.0	0.0	0.020	65	32		
	9.0	1000.0	0.0	0.020	64	31		
	9.0	1000.0	0.0	0.020	65	30		
	9.0	1000.0	0.0	0.020	64	29		
	9.0	1000.0	0.0	0.020	64	28		
	9.0	1000.0	0.0	0.020	63	28		
	9.0	1000.0	0.0	0.020	62	28		
Houma NC		10000.	100.		0.	18		0
	13.0	500.0	0.0	0.030	50	51		
	13.0	500.0	0.0	0.030	49	50		
	13.0	500.0	0.0	0.030	49	49		
	13.0	500.0	0.0	0.030	48	48		
	13.0	500.0	0.0	0.030	47	47		
	13.0	500.0	0.0	0.030	46	46		
	12.0	500.0	0.0	0.030	45	45		
	12.0	500.0	0.0	0.030	45	44		
	12.0	500.0	0.0	0.030	45	43		
	12.0	500.0	0.0	0.030	45	42		

	12.0	500.0	0.0	0.030	45	41		
	12.0	500.0	0.0	0.030	45	40		
	12.0	500.0	0.0	0.030	45	39		
	12.0	500.0	0.0	0.030	45	38		
	12.0	500.0	0.0	0.030	45	37		
	12.0	500.0	0.0	0.030	45	36		
	12.0	500.0	0.0	0.030	45	35		
	12.0	500.0	0.0	0.030	45	34		
Lafourche		10000.		100.	0.	51	0	
	12.0	800.0	0.0	0.030	61	53		
	12.0	800.0	0.0	0.030	61	52		
	12.0	800.0	0.0	0.030	61	51		
	12.0	800.0	0.0	0.030	61	50		
	12.0	800.0	0.0	0.030	60	49		
	12.0	800.0	0.0	0.030	61	48		
	12.0	800.0	0.0	0.030	61	47		
	12.0	800.0	0.0	0.030	61	46		
	6.0	800.0	0.0	0.030	61	45		
	6.0	800.0	0.0	0.030	61	44		
	6.0	800.0	0.0	0.030	60	43		
	6.0	800.0	0.0	0.030	60	42		
	6.0	800.0	0.0	0.030	60	41		
	6.0	800.0	0.0	0.030	59	41		
	6.0	800.0	0.0	0.030	59	40		
	6.0	800.0	0.0	0.030	58	39		
	6.0	800.0	0.0	0.030	58	38		
	6.0	800.0	0.0	0.030	58	37		
	6.0	800.0	0.0	0.030	58	36		
	6.0	800.0	0.0	0.030	57	35		
	6.0	800.0	0.0	0.030	56	34		
	4.0	300.0	0.0	0.030	55	34		
	4.0	300.0	0.0	0.030	54	34		
	4.0	300.0	0.0	0.030	53	34		
	4.0	300.0	0.0	0.030	52	33		
	4.0	300.0	0.0	0.030	51	32		
	4.0	100.0	0.0	0.030	51	31		
	4.0	100.0	0.0	0.030	50	30		
	4.0	100.0	0.0	0.030	49	29		
	3.0	100.0	0.0	0.030	48	28		
	3.0	100.0	0.0	0.030	47	28		
	3.0	100.0	0.0	0.030	46	28		
	3.0	100.0	0.0	0.030	45	28		
	3.0	100.0	0.0	0.030	44	28		
	3.0	100.0	0.0	0.030	43	27		
	3.0	100.0	0.0	0.030	42	26		
	3.0	100.0	0.0	0.030	41	26		
	3.0	100.0	0.0	0.030	40	26		
	3.0	100.0	0.0	0.030	39	25		
	3.0	100.0	0.0	0.030	38	25		
	3.0	100.0	0.0	0.030	37	24		
	3.0	100.0	0.0	0.030	36	23		
	3.0	100.0	0.0	0.030	36	22		
	3.0	90.0	0.0	0.030	35	21		
	3.0	90.0	0.0	0.030	35	20		
	3.0	90.0	0.0	0.030	35	19		
	3.0	90.0	0.0	0.030	34	19		
	3.0	90.0	0.0	0.030	34	18		
	3.0	90.0	0.0	0.030	35	17		
	3.0	90.0	0.0	0.030	35	16		
	3.0	90.0	0.0	0.030	36	15		
Miss R.		10000.		200000.	0.00	115	0	
	38.0	3300.0	0.0	0.030	86	59		
	38.0	3300.0	0.0	0.030	86	58		
	38.0	3300.0	0.0	0.030	87	57		
	38.0	3300.0	0.0	0.030	88	56		
	38.0	3300.0	0.0	0.030	89	55		
	38.0	3300.0	0.0	0.030	89	54		
	38.0	3300.0	0.0	0.030	90	53		
	38.0	3300.0	0.0	0.030	91	52		

38.0	3300.0	0.0	0.030	91	51
38.0	3300.0	0.0	0.030	92	50
38.0	3300.0	0.0	0.030	92	49
42.0	2500.0	0.0	0.030	91	48
51.0	2500.0	0.0	0.030	91	47
62.0	2100.0	0.0	0.030	90	46
85.0	2500.0	0.0	0.030	89	45
58.0	2500.0	0.0	0.030	88	44
63.0	2600.0	0.0	0.030	88	43
100.0	2500.0	0.0	0.030	87	42
100.0	2500.0	0.0	0.030	86	42
100.0	2500.0	0.0	0.030	85	42
100.0	2200.0	0.0	0.030	84	42
100.0	2100.0	0.0	0.030	83	42
90.0	2300.0	0.0	0.030	81	41
90.0	2000.0	0.0	0.030	81	40
85.0	1300.0	0.0	0.030	81	39
60.0	1300.0	0.0	0.030	80	38
100.0	1700.0	0.0	0.030	79	38
100.0	2000.0	0.0	0.030	78	38
100.0	2500.0	0.0	0.030	77	37
85.0	2500.0	0.0	0.030	77	36
80.0	2300.0	0.0	0.030	76	36
75.0	2800.0	0.0	0.030	75	35
85.0	2500.0	0.0	0.030	74	34
85.0	2500.0	0.0	0.030	73	33
90.0	2400.0	0.0	0.030	72	33
70.0	2200.0	0.0	0.030	71	33
85.0	2500.0	0.0	0.030	70	32
95.0	2000.0	0.0	0.030	69	31
80.0	2000.0	0.0	0.030	69	30
90.0	2500.0	0.0	0.030	69	29
90.0	2500.0	0.0	0.030	68	29
100.0	2500.0	0.0	0.030	67	28
100.0	2500.0	0.0	0.030	67	27
83.0	2300.0	0.0	0.030	67	26
90.0	2500.0	0.0	0.030	68	25
80.0	2500.0	0.0	0.030	68	24
100.0	2500.0	0.0	0.030	69	23
100.0	2500.0	0.0	0.030	70	23
95.0	2200.0	0.0	0.030	71	23
65.0	2700.0	0.0	0.030	71	22
100.0	2200.0	0.0	0.030	70	21
100.0	2000.0	0.0	0.030	69	21
70.0	2700.0	0.0	0.030	68	21
100.0	2200.0	0.0	0.030	67	21
90.0	2300.0	0.0	0.030	66	21
70.0	1900.0	0.0	0.030	66	22
70.0	1900.0	0.0	0.030	65	22
70.0	2300.0	0.0	0.030	64	22
70.0	2300.0	0.0	0.030	63	22
70.0	2000.0	0.0	0.030	63	21
100.0	2000.0	0.0	0.030	62	21
60.0	2400.0	0.0	0.030	61	21
75.0	2000.0	0.0	0.030	61	20
70.0	2000.0	0.0	0.030	60	20
70.0	2100.0	0.0	0.030	59	20
90.0	1700.0	0.0	0.030	58	21
100.0	1700.0	0.0	0.030	57	21
75.0	2100.0	0.0	0.030	56	21
70.0	2000.0	0.0	0.030	55	20
75.0	2900.0	0.0	0.030	54	19
100.0	1900.0	0.0	0.030	53	19
100.0	1600.0	0.0	0.030	53	18
70.0	2000.0	0.0	0.030	52	17
100.0	2000.0	0.0	0.030	51	17
70.0	2300.0	0.0	0.030	50	17
95.0	1900.0	0.0	0.030	49	17
75.0	2000.0	0.0	0.030	48	18

55.0	2000.0	0.0	0.030	47	17
55.0	2000.0	0.0	0.030	46	18
55.0	2000.0	0.0	0.030	46	18
65.0	2500.0	0.0	0.030	45	18
100.0	2000.0	0.0	0.030	44	18
95.0	2400.0	0.0	0.030	43	19
85.0	2100.0	0.0	0.030	42	20
95.0	2500.0	0.0	0.030	41	19
95.0	2500.0	0.0	0.030	41	18
55.0	2000.0	0.0	0.030	41	17
80.0	1700.0	0.0	0.030	40	17
85.0	2000.0	0.0	0.030	39	16
85.0	1800.0	0.0	0.030	39	15
100.0	2000.0	0.0	0.030	38	14
50.0	3200.0	0.0	0.030	37	15
65.0	2400.0	0.0	0.030	36	15
100.0	2000.0	0.0	0.030	35	15
100.0	2000.0	0.0	0.030	35	14
90.0	2200.0	0.0	0.030	36	14
90.0	2000.0	0.0	0.030	36	13
40.0	2800.0	0.0	0.030	35	12
55.0	2000.0	0.0	0.030	34	11
40.0	2400.0	0.0	0.030	33	12
53.0	2200.0	0.0	0.030	32	12
40.0	3100.0	0.0	0.030	31	12
55.0	2500.0	0.0	0.030	31	11
70.0	2800.0	0.0	0.030	32	11
85.0	2000.0	0.0	0.030	32	10
55.0	2400.0	0.0	0.030	31	9
55.0	2800.0	0.0	0.030	30	9
60.0	2800.0	0.0	0.030	29	8
80.0	1600.0	0.0	0.030	29	7
65.0	3500.0	0.0	0.030	28	6
40.0	2900.0	0.0	0.030	29	5
50.0	2500.0	0.0	0.030	30	4
40.0	2400.0	0.0	0.030	30	3
40.0	2400.0	0.0	0.030	30	2
70.0	2400.0	0.0	0.030	30	1
MRGO	10000.	100.	0.	31	0
40.0	600.0	0.0	0.030	96	37
40.0	600.0	0.0	0.030	95	36
40.0	600.0	0.0	0.030	94	35
40.0	500.0	0.0	0.030	93	34
40.0	500.0	0.0	0.030	92	34
40.0	500.0	0.0	0.030	91	33
40.0	500.0	0.0	0.030	90	32
40.0	500.0	0.0	0.030	89	31
40.0	500.0	0.0	0.030	88	31
40.0	500.0	0.0	0.030	87	30
40.0	500.0	0.0	0.030	86	29
40.0	500.0	0.0	0.030	85	28
40.0	500.0	0.0	0.030	84	28
40.0	500.0	0.0	0.030	83	27
40.0	500.0	0.0	0.030	82	26
40.0	500.0	0.0	0.030	81	25
40.0	500.0	0.0	0.030	80	24
40.0	500.0	0.0	0.030	79	24
40.0	500.0	0.0	0.030	78	24
40.0	500.0	0.0	0.030	77	24
40.0	500.0	0.0	0.030	76	23
40.0	500.0	0.0	0.030	75	22
40.0	500.0	0.0	0.030	74	22
40.0	500.0	0.0	0.030	73	21
40.0	500.0	0.0	0.030	72	20
40.0	500.0	0.0	0.030	71	19
40.0	500.0	0.0	0.030	70	19
40.0	500.0	0.0	0.030	69	19
40.0	500.0	0.0	0.030	68	19
40.0	500.0	0.0	0.030	67	19

.0 40.0 .0 500.0 .0 0.0 .0 0.030 67 20
0 0 .0 .0

Sensitivity Runs

Several surge simulations were computed to determine the sensitivity of the final water elevations to variations in the input parameters. The sensitivity runs were made using the offshore and inland grids and various combinations of hurricane parameters, barrier locations and elevations, and roughness parameters. The final offshore calibration run uses the values of these parameters that produced the best agreement between the observed surge elevations and the predicted surge elevations at shoreline locations near the transfer grid cells.

The inland sensitivity runs involved varying several parameters; the position of: the hurricane, the river depth turbidity, and roughnesses; the ridge elevations and locations; the overland roughness and the tide level. In order of overall importance, the barrier elevation changes had the greatest effect on the still water elevations, followed by the overland roughness and by the tide level. The sensitivity of the calibration computer runs to various values of the land Manning roughness were investigated in detail. Variable overland roughness was used in several simulations and did not produce a significant difference from constant roughness simulations.

Tide Calibration

The calibration of inland grid was conducted using observed tide range and elevation data for various locations along the Louisiana coast and along waterways. Tide records from continuously recording tide gauges were obtained from the New Orleans District office of the U. S. Army Corps of Engineers. These tide gauges in many cases are at the same locations as gauges which have recorded hurricane surge elevations.

The tidal calibration for the chenier plain focussed on the Calcasieu ship channel. Three tide gauges were used. The first gauge (No. 73650) was located at the mouth of the Calcasieu ship channel south of the city of Cameron. The second gauge was located at Hackberry (No. 73600) about 20 miles north of Cameron. The third gauge was located south of Lake Charles (No. 73550) about 45 miles north of Cameron. The predicted elevations and time lags are comparable to the observed elevations at Cameron and Hackberry. The predicted elevation at Lake Charles was low by about .8 ft. This location is most affected by backwater in the Calcasieu Ship Channel and the operation of the Calcasieu Lock. The tidal ranges, based upon the

falling high tide is well predicted at Cameron, but is over-predicted at Hackberry and Lake Charles.

The tidal calibration for the deltaic plain focussed on the Barataria Bay and Lake Pontchartrain basins. Tide gauge records for several locations in both basins were used. Tide gauge locations used in the Barataria Basin were Grand Isle (88410), Bayou Petit Caillou (76305), Bayou Blue (82301), Bayou Barataria at Lafitte (82875), Bayou Barataria at Barataria (82750), Houma (76320), Bayou Des Allemands at Des Allemands (82700), Bayou Chevrenul at Chegby (82525), and Greenwood (52880). In the Pontchartrain Basin, the tide gauges used were Seabrook Bridge (76060), Mississippi River Gulf Outlet at Shell Beach (85800), Mandeville (85575), West End (85625), Mid-lake (85600), Regolets (857001), and Irish Bayou (85675). The tidal calibration simulations showed good agreement in tidal range throughout both basins, however, the predicted tidal crest elevations were lower than the actual elevations. This was because there were mean water changes in the northern parts of the two basins that were related to wind tides and run-off outside of channels that are not accounted for in the model.

Hurricane Calibration

The hurricane calibration was conducted for five storms; Audrey, Carla, Betsy, Camille and Andrew. These storms were selected to give a good geographic coverage of the state, even though several are dated. More recent storms, i.e., Juan, Gilbert, and Frederick, were reviewed but since they produced little overland flooding in coastal Louisiana they were not useful. In order to properly calibrate the simulation model, the conditions which existed at the time of the hurricane would have to be reproduced. Some of the barriers in existence at the present were either absent or had a reduced height in the past, river channels have deepened, several roadways have been raised, and marsh conditions have changed. The approach taken in the calibration effort was to use the present data base for the calibration simulations.

Hurricane Andrew is the most appropriate storm to be used for calibration because it has recently occurred and reflects the current landscape conditions in the state. The storm data has been obtained from Rappaport [2] and Martin [3]. The central pressure reached a low of 937 millibars after crossing into the Gulf of Mexico. Calibration of the storm involved using 17 data points for which either gauge data or high water marks were available. The comparison of the observed and computed maximum surge elevations showed an average different of -.5 feet and an RMS

difference of 1.5'. Thus, the model slightly underpredicted the maximum surge elevations. This occurred near the point of landfall where the wind direction shifted 180° as the storm passed.

Hurricane Audrey is the most appropriate storm for calibrating the chenier plain. The storm had a low central pressure and relatively constant pressure, radius, forward velocity, and direction. It produced water levels that were at or exceeding the 100-year elevation. There are several gauge and overland water level observations available. Hurricane Carla was used as a calibration storm, although it did not produce significant ridge overtopping and therefore could only be used to calibrate river and waterway flooding. The calibration data points and geographic locations for Audrey were taken from [4] and [5]. These reports list each observation, its location, and the source of the data. The gauge data stations are located on waterways and can be expected to be strongly influenced by waterway characteristics. This may not necessarily reflect overland water levels, even in close proximity. The high water mark data is extensive and covers the full extent of the parish. Some of the data appears to be affected by both wave action and the exposure of the site. The calibration of the model was accomplished using the observed data to control both the offshore and inland computations. The surge elevations vary an average of about 1 foot for a Manning's change from .033 to .039. The offshore calibration results show a good agreement with the observed surge elevations at the grid points near the boundary transfer to the inland grid. The agreement between the observed and predicted surge elevations for the inland simulation are excellent. There is no average difference and the rms difference was .5 feet. The agreement between observed and predicted surge elevations is very good at critical locations within the parish. At the coast at the Coast Guard station, the observed was 12.1 ft, while the predicted is 12.8. The elevation differences at the Calcasieu Locks is also predicted, i.e., the predicted elevation is 7.6 ft at the West Lock compares to the 7.7 observed. The predicted elevation at the East Lock is 5.9 ft with an observed value of 5.5 ft. The predicted elevation at the Hackberry gauge is 6.4 ft while the observed is 6.7 ft. The still water elevation prediction at the head of Grand Lake is 4.8 ft while the observed value was 5.5 ft. After being calibrated for hurricane Audrey, the surge model was used without changes to the input topographic data.

JOINT PROBABILITY SIMULATIONS

The joint probability (JP) computer runs were conducted using control software developed particularly for the study. The FEMA surge program was re-written to take input files consisting of the depth data, the hurricane data, and the input file. The multiple executions of the surge model were controlled by the batch file program. This batch file executes a Fortran program for each of the JP runs. The JP files for each run are identified by indicating each run parameter, so that the P1R1V1D1.1 represents the first pressure, radius, velocity, direction, and track. The alongshore JP runs are designated as being a fourth direction, i.e., D4.

Set-Up of the Joint Probability Runs

The joint probability runs were based upon the hurricane discretization described in the previous section. The hurricane tracks were set-up such that the track was constrained to pass through fixed points along the coastal line. The control points were along a latitude of 29.75 degrees and were separated by .500 degrees in longitude starting at a longitude of 89.00 and extending westward to 95.0 degrees. The track separation is about 25 nautical miles or about equal to the radius of a hurricane storm. The extreme eastward and westward limits were set based on producing a maximum shoreline surge elevation of less than 8 feet. A total of 408 simulations were run. The output of the maximum elevations of the surge for the inland runs were saved as a maximum water elevation file, with extension MX, i.e., P1R1V1D1.1MX.

Still Water Elevations

The final still water exceedence probabilities for each of the inland grid cells were calculated by summing all the MX files, weighing each elevation with the appropriate probability. Thus, for each cell, the exceedence probability statistics were calculated. From these statistics, the water elevation for a fixed annual probability of rise in water level could be found. Thus, for an annual exceedence probability of .01, the elevation was interpolated from the exceedence statistics. The annual probability of .01 corresponds to an average return period of 100 years. The water elevations

having annual exceedence probabilities of .002, .02, .04, and .1 were also interpolated for the average return periods of 500, 50, 25, and 10 years respectively.

The final flood elevations for each cell for average return period of 10, 25, 50, 100, and 500 years are presented in the user manual, Volume II of this report. The manual contains instructions for the use of the table.

DISCUSSION OF RESULTS

Data Base

The data used in the study were obtained from a variety of sources having different dates. These data are critical to the accurate prediction of hurricane flood levels in coastal Louisiana. These data were not all checked during the calibration phase of the study. Calibration is particularly sensitive to the hydraulic properties of the rivers and channels in the study area, both because the calibration data is taken in waterways and because all stages of flooding effect waterway water elevations. The ground elevation and roughness properties of marsh areas of the study area could not be as well documented as barrier and river data, nor were there many observations of flooding in marsh areas. Thus, while the marsh areas comprise the vast majority of the area in the study site and have an important effect on flooding, it is the least accurately determined component of the data base.

Model

The surge model used appears to be very well suited to the purposes of this study. It was capable of accommodating the significant landscape features of coastal Louisiana and performed well. Certain limitations of the model exist which could have had a small effect on the computed flood elevations.

The model limits the number of grid cells, barriers, and rivers that can be included in the computations. This prevented some small sub-grid scale features from being included in the modeling. Instabilities during simulations developed during the set-up of the sensitivity runs that required re-assignment of some land elevations near rivers. No difficulties were encountered in the calibration or production runs once these changes were made.

Use of the Model

The set up of the grids based upon the Lambert coordinates was very convenient and allowed the grids to be referenced on virtually any topographic maps. The NOS charts, however, do not include Lambert tick marks and so the use of these charts required manual plotting of Lambert coordinates. The grid sizes used appear to be adequate for the purpose of simulating extreme hurricane surges. For less severe storms, water movement would be influenced by smaller channels than could be represented at the 10,000-foot resolution.

CONCLUSIONS AND RECOMMENDATIONS

Based upon the data base and methodology used in this study it can be concluded that:

1. The hurricane flood elevations in Louisiana indicate that flooding of transportation facilities in coastal Louisiana will be severe enough to require incorporation into design of new facilities.
2. The landscape changes taking place in coastal Louisiana are of sufficient magnitude to modify the threat of hurricane flooding to transportation facilities in this area.

It is recommended that the predicted flood elevations statistics be periodically updated by recomputing the flood statistics using new data as it becomes available. The new data should include new landscape features, such as highways, levees and channels. Also any major changes in the landscape of the coastal zone of Louisiana that are now being planned by the Coastal Restoration Division of DNR will certainly have an effect on hurricane flood elevations and should be incorporated into the model. This update would involve preparing a new users manual which would need to be distributed to the appropriate LaDOTD offices.

It is also recommended that the acquisition of new data for the marsh areas of the coast be initiated and that the data be incorporated into the data base for the model. Specifically, this would include obtaining marsh water level and ground level data referenced to a suitable datum such as the latest NGVD. Data could routinely be obtained from professional surveyors who in the course of their work would survey marsh areas. Also, several federal, state, and local governmental agencies are involved in monitoring water levels within marsh areas. In particular, the Coastal Restoration Division of the Department of Natural Resources is instrumenting several marsh sites in coastal Louisiana with tide gauges. The data from these sources of opportunity would be very useful in the future for updating the hurricane flood elevation statistics.

REFERENCES

1. Ho, Frances P., James C. Su, Karen L. Hanevich, Rebecca J. Smith, and Frank Richards, Hurricane Climatology for the Atlantic and Gulf Coasts of the United States, NOAA, Water Management Division, Office of Hydrology, National Weather Service, Draft, (abstract), July 3, 1986.
2. Rappaport, Ed, Hurricane Andrew - A Preliminary Look, Mariners Weather Log, Fall, p. 16-25.
3. Martin, E, Hurricane Andrew Data for Coastal Louisiana, Personal Communication, February 1993.
4. Corps of Engineers, Hurricane Audrey - 27 June 1957, Memorandum Report, New Orleans District, September 30, 1960.
5. Crawford, Kenneth, Hurricane Surge Potentials Over Southeast Louisiana as Revealed by a Storm-Surge Forecast Model: A Preliminary Study, National Weather Service, New Orleans, LA., Vol. 60, No. 5, May, 1979, 422-428 pp.

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